

CHAPTER 2: Materials

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FUNCTIONAL REQUIREMENT

2.1 TIMBER

Workmanship

- i. All workmanship must be within defined tolerances as defined in Chapter 1 of this Manual.
- ii. All work to be carried out by a technically competent person in a workmanlike manner.

Materials

- i. All materials should be stored correctly in a manner which will not cause damage or deterioration of the product.
- ii. All materials, products and building systems shall be appropriate and suitable for their intended purpose.
- iii. External timber should be adequately treated or finished to resist insect attack. Timber treatment should be in accordance with relevant British Standards and Codes of Practice.
- iv. The structure shall, unless specifically agreed otherwise with the warranty provider, have a life of not less than 60 years. Individual components and assemblies, not integral to the structure, may have a lesser durability but not in any circumstances less than 15 years.
- v. Timber used in dwelling to provide support to the structure must be appropriately seasoned to prevent excessive shrinkage and movement.

Design

- i. Design and specifications shall provide a clear indication of the design intent and demonstrate a satisfactory level of performance.
- ii. Structural elements outside the parameters of Approved Document A (England and Wales); Section 1 (Scotland) and Technical Booklet D (Northern Ireland) must be supported by structural calculations provided by a suitably qualified expert.
- iii. The materials used for construction must meet the relevant Building Regulations and other statutory requirements:
 - The Building Regulations 2010
 - The Building (Scotland) Regulations 2011
 - The Building (Northern Ireland) Regulations 2010
 - British Standards and Euro-Codes (refer to Appendix 2a for a list of standards referred to).
- iv. Specialist works must be provided and supported by structural calculations completed by a suitably qualified engineer where necessary.
- v. Any off-site manufactured engineered beams / posts must have structural calculations which have been endorsed by the manufacturer.

2.1.1 Storage

Timber should be stored correctly to ensure that it does not deteriorate. Timber should be kept dry and covered in cold conditions to prevent surface freezing. It should be kept off the ground and spaced to allow air to move around freely. Timber should be kept flat to prevent warping or twisting.

2.1.2 Timber durability

Timber should be appropriately treated to resist insect attack. Some timber species have a natural ability to resist attack. Table 1 identifies various species of timber and whether treatment is required.

2.1.3 Timber grading

Timber should be of the appropriate strength classification in order to meet its design intention. For timber that is to be used for structural purposes, e.g. floor joists, rafters and ceiling joists, the strength classification should be assumed to be C16 unless it is appropriately stamped with its strength classification.

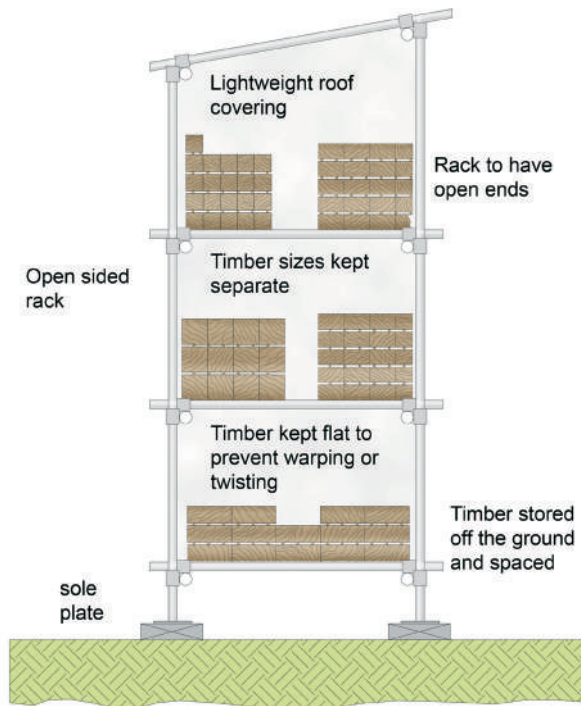


Figure 1: Storage of timber on-site

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Durability class	Timber type	Species	Variety	Typical strength grade*
Very durable	Softwood	None		
	Hardwood	Opepe Padauk- Andaman Afromosia Greenheart Guarea Iroko Jarrah Okau Pyinkado Teak Kapur Padauk Peroba	Malaysian Sabah Burma White	D50 N/A N/A D70 N/A D40 D40 N/A N/A D40 D60 D60 N/A N/A
Durable	Softwood	Cedar	Western red (non UK)	C18
	Hardwood	Besralocus Ekki Chestnut Karri Kampas Louro Oak Mahogany	Sweet Red American White European American	N/A D70 N/A D50 N/A D50 D30 N/A
Moderately durable	Softwood	Pine	Caribbean Pitch American Pitch	C24 N/A
		Cedar	Western Red (UK)	C18
		Fir	Douglas (North America) Douglas (UK) Dunkeld (UK)	C16-C24 C18 N/A
		Larch	European Hybrid Japanese Tamarack Western Maritime	C16-C30 C16-C30 C16-C30 C16-C30 C16-C30 C16-C30

Durability class	Timber type	Species	Variety	Typical strength grade*
Moderately durable	Hardwood	Keruing	Sabah	D50
		Oak Mahogany	Malaysian Tasmanian Turkey African	D50 N/A N/A N/A
Slightly durable	Softwood	Fir	Noble Silver Balsam Grand	C16-C24 C16-C24 C16-C24 C16-C24
		Pine	Canadian Red Corsican Jack Parana Ponderosa Radiata Scots Southern Western White Yellow Lodgepole	C16 C16 C16 C16 C16 C16-C24 C16-C30 C16 N/A N/A
	Hardwood	Redwood Hem-fir	European USA and Canada	C16-C24 C16 C16
		Spruce	Eastern Canadian Engelmann European (whitewood) Sitka Western White Canada	C16 C16 C16 C16 C16-C24
	Hardwood	Elm	Dutch English White Rock Wych American Red Silver	N/A N/A N/A N/A D40 D35-D40

Durability class	Timber type	Species	Variety	Typical strength grade*
Not durable	Softwood	None		
	Hardwood	Alder Beech Birch	European	N/A
Silver			D35-D40	
Birch			N/A	
Chestnut		European	N/A	
		Paper	N/A	
		Yellow	N/A	
Lime Sycamore	European	N/A		
	Horse	N/A		

* Denotes typical strength grade and is for guidance purpose only.

Table 1: Characteristics of timber species

2.1.4 Timber treatment

Timber should ideally be preserved in a factory environment, it is accepted however, that this is not always possible. Timber treatments should be appropriately approved to the relevant Code of Practice, British Standard or have third party accreditation. Careful consideration should be given to Health and Safety when applying timber treatment products. It is important that any pre-treated timber should be re-treated if it is cut and exposes untreated end grain. The treatment should be coloured so that it can be proven that the end grain has been treated.

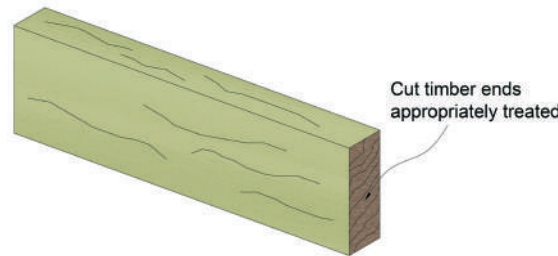


Figure 2: Pre-treated timber exposing un-treated end grain.

2.1.5 Metal fixings

Metal components should be galvanised where they are to be fixed or used adjacent to treated timber.

2.1.6 Standards referred to

- BS EN 1912 2004+A4:2010 Structural timber-strength classes-assignment of visual grade and species
- BS EN 1995 1 1 2004 and 2008 Euro-Code design of timber structures
- BS EN 5999-Part 1 Durability of wood and wood based products

FUNCTIONAL REQUIREMENT

2.2 CONCRETE

Workmanship

- i. All workmanship must be within defined tolerances as defined in Chapter 1 of this Manual.
- ii. All work to be carried out by a technically competent person in a workmanlike manner.
- iii. Concreting shall not take place during cold weather periods or where ground conditions are frozen.

Materials

- i. All materials should be stored correctly in a manner which will not cause damage or deterioration of the product.
- ii. All materials, products and building systems shall be appropriate and suitable for their intended purpose.
- iii. The structure shall, unless specifically agreed otherwise with the warranty provider, have a life of not less than 60 years. Individual components and assemblies, not integral to the structure, may have a lesser durability but not in any circumstances less than 15 years.

Design

- i. Design and specifications shall provide a clear indication of the design intent and demonstrate a satisfactory level of performance.
- ii. Structural elements outside the parameters of Approved Document A (England and Wales); Section 1 (Scotland) and Technical Booklet D (Northern Ireland) must be supported by structural calculations provided by a suitably qualified expert.
- iii. The materials used for construction must meet the relevant Building Regulations and other statutory requirements:
 - The Building Regulations 2010
 - The Building (Scotland) Regulations 2011
 - The Building (Northern Ireland) Regulations 2010
 - British Standards and Euro-Codes (refer to Appendix 2a for a list of standards referred to).
- iv. Reinforced concrete elements must be supported by structural calculations and details produced by a suitably qualified Structural Engineer.
- v. Pre-cast structural elements must have structural calculations which prove their adequacy that have been endorsed by the manufacturer.

2.2.1 Cold weather working

To meet the Functional Requirements of this Chapter, minimum working temperatures should not fall below 2°C. It is important that during cold weather periods, regular temperature readings should be taken. Thermometers should be placed away from direct sunlight, preferably in a shaded area. When assessing the temperature it is important to also consider wind chill and weather exposure and make the necessary allowances for sites that have a higher level of exposure.



Figure 3: Cold weather working

2.2.2 Ready mixed concrete

It is a requirement of BS 5328 that ready mixed concrete should not be delivered if the outside temperature is below 5°C.

2.2.3 Site mixed concrete

Site mixing is acceptable at low temperatures subject to;

- The minimum temperature to be no less than 2°C
- An appropriate level of protection of the concrete during curing time
- Ground conditions are not frozen

2.2.4 Concreting of foundations and over-site

Concrete should not be poured if the ground is frozen because frozen ground can change in stability and volume during thawing and therefore may cause damage to the recently poured concrete.

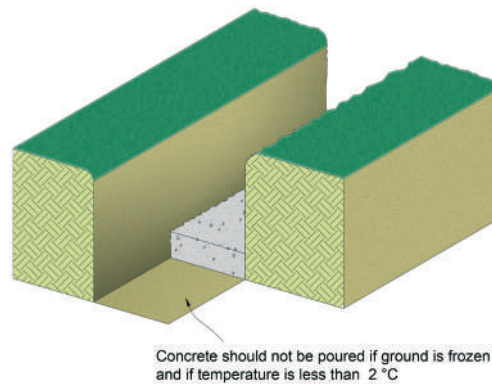


Figure 4: Concrete pouring on cold weather conditions

During cold weather, it may be appropriate to cover the ground to prevent freezing and in some extreme cases, heating of the ground may be required.

Concrete reinforcing and formwork should not be frozen and be free from snow and ice.

2.2.5 Curing of concrete

Concrete may take longer to cure during cold conditions. An additional six days may be required in extreme cases. Concrete may be covered with a rigid insulation to prevent freezing during curing periods. This is particularly useful for oversized slabs.

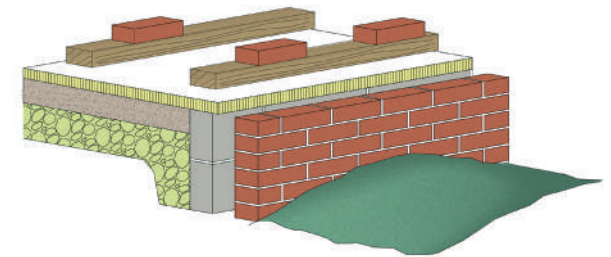


Figure 5. Concrete curing on cold weather conditions.

2.2.6 Concrete suitability

Concrete of the appropriate durability and strength should be used in all circumstances. Table 2 gives details of the correct concrete for varying applications.

Application	Ready mixed concrete	Site mixed concrete	Consistence class
Substructure Blinding (unreinforced) Backfilling	GEN1	N/A	S3
Substructure (non reinforced) Structural blinding Strip, trench and mass filled foundations Concreting of cavity walls to ground level	GEN1	N/A	S3/S4
Floor (dwellings unreinforced and unsuspended)	GEN1	N/A	S2
With screed added or other floor finish Floor slab as finish (e.g. power float)	GEN2	N/A	S2
Garage floors (unreinforced and unsuspended)	GEN3	N/A	S2
Reinforced slabs (dwellings and garages suspended or unsuspended)	RC35	N/A	S2
Superstructure	As specified by Structural Engineer	N/A	As specified by a Structural Engineer
External works Pathways Bedding for paving slabs	PAV1 GEN1	ST5 ST1	S2 S1

Table 2: Concrete suitability

2.2.7 Concrete mixes

2.2.7.1 Ready mixed concrete

Concrete must be mixed using the correct proportions of cement, sand, aggregate and water. Ready mixed concrete should be delivered as close as possible to the site works and should be poured immediately to prevent settlement or separation of the mix. Ideally, ready mixed concrete should be poured within two hours of the initial mixing at the concrete plant.

Ready mixed concrete should only be sourced from a supplier who has a quality control system in place to ensure the correct standard of concrete is delivered. The quality control scheme should be either QSRMC (the Quality Scheme for Ready Mixed Concrete) or a relevant BS Kitemark scheme.

It is important that all design specifications of the concrete are passed to the ready mixed supplier to ensure that the delivered concrete meets the design intention.

Delivery notes should be kept and made available for inspection if required.

Additional water should not be added to the concrete on-site nor should the ready mixed

concrete be poured into water filled trenches unless the concrete has been specifically designed for this purpose.

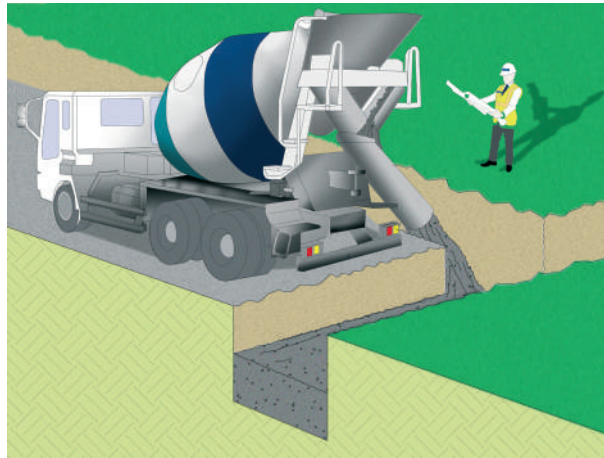


Figure 6: Ready mixed concrete

2.2.7.2 Site mixed concrete

Site mixed concrete should generally be avoided unless it is for non-structural applications, e.g., backfilling or bedding of paving slabs etc. There may be exceptional circumstances where site mixing is unavoidable. Where this is the case, extra caution must be taken to ensure that the correct mix proportion is used, delivery notes should be provided if necessary and provision for testing may be required.

2.2.7.3 Concrete reinforcing

Reinforcing bars and mesh should be clean and free from loose rust and any other contaminants which may cause deterioration of the reinforcing or the durability of the concrete.

Reinforcing bars and mesh should be placed in accordance with structural drawings; bars that are to be bent should be done so using the correct tools for the job.

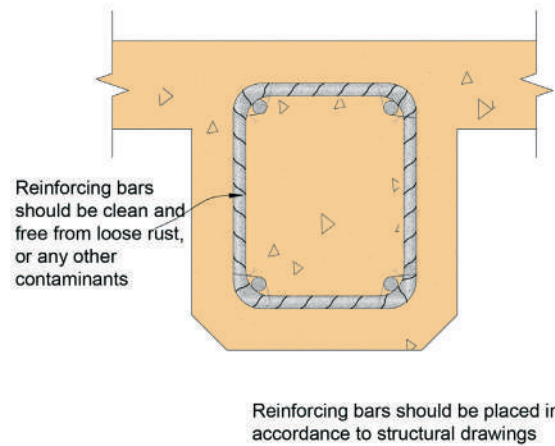


Figure 7: Reinforcing bars in concrete beam

Reinforcing bars should be correctly positioned ensuring that there is appropriate concrete cover and reinforcing mesh is placed in the right direction (main bars parallel with span).

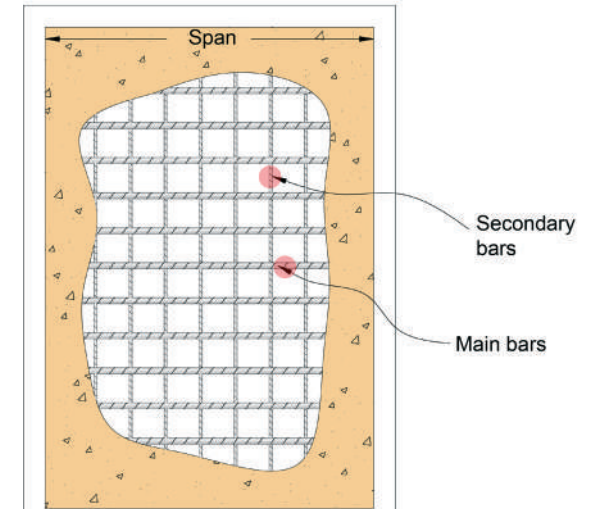


Figure 8: Position of bars on reinforced concrete slab

2.2.8 Reinforcing cover

An appropriate level of concrete cover should be provided to the reinforcing, the cover thickness will depend on the exposure of the concrete and its application. Concrete cover should be specified by a qualified Structural Engineer or alternatively using Table 3.

Application (concrete position)	Minimum cover (mm)
Concrete in direct contact with the ground	75
All external applications e.g. shuttered walling	50
Floor slabs and other applications where concrete is cast onto a membrane	40
Concrete over blinding concrete	40
Internal conditions	25

Table 3: Minimum concrete reinforcing cover

Reinforcing should be supported off propriety chairs or spacers and can be made of concrete, plastic or steel. The thickness and depth of a concrete spacer should not exceed 50mmx50mm. Spacers should be placed at maximum 1m centres and when supporting mesh should be staggered

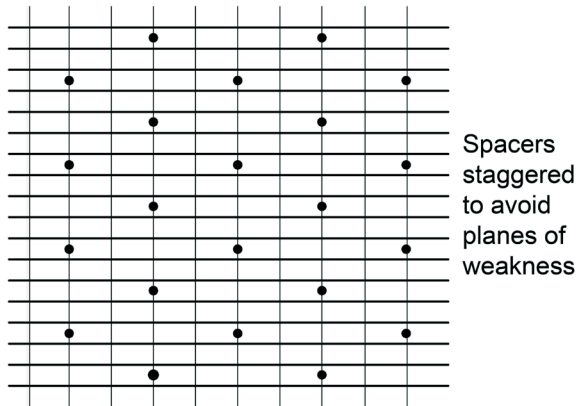


Figure 9: Position of spacers

2.2.9 Admixtures

Admixtures should only be used if specified as part of the original design specification. If an admixture is to be proposed where it was not intended to be as part of the design, confirmation by a Structural Engineer that the admixture is appropriate is required.

It is important that the appropriate amount of admixture is applied to any mix. Any overdosing may cause concrete deterioration or poor workability.

2.2.9.1 Common admixtures

Plasticisers: improve the workability of concrete especially when pumped; they can also improve concrete adhesion which is enhanced particularly when concrete is reinforced.

Air entraining agents: increase the air void volume of concrete which in turn produces a surface more resilient to cold weather, and is therefore ideally suited to outdoor conditions where cold weather exposure is high; such as pathways or roads.

Accelerators: provide an improved curing time-caution should be taken to allow for reasonable time to “finish” the concrete.

2.2.9.2 Admixtures in cold weather

Admixtures may be used in cold weather, but usually will not assist in preventing concrete from freezing and therefore should not be relied upon to

compensate for freezing conditions. The guidance for cold weather working should be followed in these circumstances.

2.2.9.3 Admixtures and reinforcing

Admixtures containing chloride will cause corrosion to occur and should not be used in concrete containing reinforcing.

2.2.10 Expansion / movement joints

Joints in concrete should be provided to prevent cracking caused by shrinkage; shrinkage will be less significant if the concrete is reinforced. Table 3 gives details of maximum areas of concrete permissible without expansion joints.

An increased amount of expansion joints should be provided to concrete where weak spots may occur. This may include a narrowing width of floor slab for example.

2.2.11 Vibration and compaction of concrete

Reinforced concrete should be compacted using a vibrating poker, care must be taken to ensure that the concrete is not over compacted and the concrete mix separated. Tamping of floors by hand is acceptable for floor slabs that do not exceed 150mm in thickness.

2.2.12 Curing of concrete

Concrete should be adequately cured before loads are applied. It is acceptable that masonry to Damp Proof Course (DPC) is built onto a foundation that

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is not fully cured, however care must be taken to prevent any damage to the foundation. The concrete should be at least durable enough to carry the masonry.

The speed at which concrete mixes cure depend on the mix ratio and whether there are any additives within the concrete. Where curing time is critical such as cast in-situ upper floors, curing times should be indicated as part of the design and formwork struck as advised by a Structural Engineer.

To prevent concrete curing too rapidly after initial drying exposed concrete should be covered with hessian, polythene or sand. This prevents the surface drying too quickly and protects the concrete surface. This level of protection is particularly critical in hot or adverse weather conditions.

Standards referred to:

- BS 8110 Structural Use of Concrete
- BS EN 12620 Aggregates for Concrete
- BS EN 197 Cement. Conformity Evaluation.

FUNCTIONAL REQUIREMENT

2.3 OTHER COMPONENTS

Workmanship

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Materials

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- ii. Structural elements outside the parameters of Approved Document A (England and Wales) Section 1 (Scotland) and Technical Booklet D (Northern Ireland) must be supported by structural calculations provided by a suitably qualified expert.
- iii. The materials used for construction must meet the relevant Building Regulations and other statutory requirements, British Standards and Euro-Codes.

2.3.1 Cold weather working

To meet the Functional Requirements of this Chapter, minimum working temperatures should not fall below 2°C when working with masonry. It is important that during cold weather periods, regular temperature readings should be taken.

Thermometers should be placed away from direct sunlight, preferably in a shaded area. When assessing the temperature, it is also important to consider wind chill and weather exposure, making necessary allowances for sites that have a higher level of exposure.

2.3.1.1 Protection of materials

Covers should be provided to protect materials from frost, snow and ice, in particular bricks, blocks, sand and cement. Frozen materials should never be used in any circumstances.

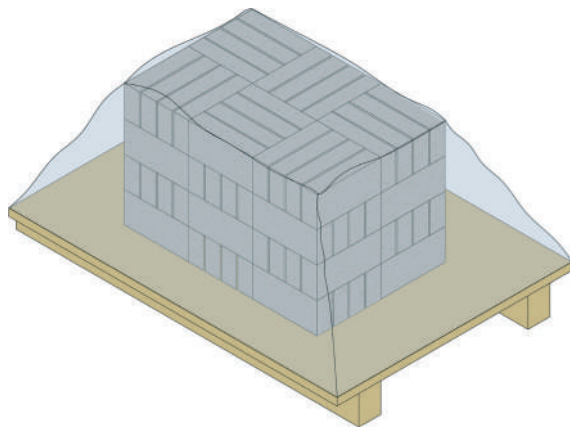


Figure 10: Protection of blockwork

2.3.2 Protection of masonry

Any new walls or other masonry construction will require protection against frost where temperatures are expected to drop below 2°C. Ideally, all masonry should be protected with polythene or hessian. If temperatures are expected to fall to an extremely low level, insulation boards may also be required or even heating may be considered.

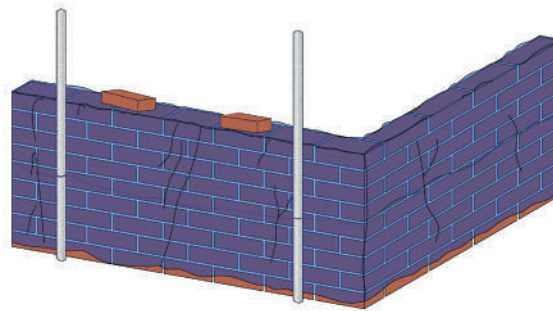


Figure 11: Protection of masonry walls

Rendering should only be completed if the outside temperature is at least 2°C. There should be no frost within the construction that is to be rendered and were possible, rendering should not take place where it is anticipated that freezing weather conditions will occur before the render has adequately cured.

No plastering or screeding should take place unless the building is free from frost. It is acceptable to use heating internally to warm the building effectively, however it is important

to ensure that heaters do not emit excessive vapour into the dwelling. Adequate ventilation should be provided to allow moist air to escape. The dwelling should be appropriately pre-heated before plastering and continue to be heated whilst the plaster dries.

2.3.3 Masonry

Bricks should be of an appropriate durability to meet the design intention. The type of brick to be used will affect the specification of the mortar. Bricks with higher durability should be used where there is a higher potential of saturation or severe exposure to wind driven rain.

Durability	Frost resistance	Soluble salts content
FL FN	Frost resistant durable in all uses	Limits of soluble salts are defined by tests
ML MN	Moderately frost resistant, durable except when saturated and subject to repeated freezing and thawing	Low (L) Normal (N)
OL ON	Not frost resistant. Bricks liable to be damaged by repeating freezing and thawing. For internal use only	
Note: calcium silicate and concrete bricks contain no soluble salts		

Table 4: Durability of brickwork

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Use	Brick type			Notes on mortar
	Clay	Calcium Silicate	Concrete	
Foundation to DPC	FL, FN, ML, MN	Class 3	Strength >20N/mm ²	
Foundation to DPC, (sulphates in soils)	FL, FN, ML, MN	Class 3	Strength > 20N/mm ² , all Class 1 sulphates and in some Class 2, consult manufacturers. Engineering quality concrete bricks up to Class 3 sulphates	Where sulphates are Class 3 or higher use sulphate resisting Portland cement.
Un-rendered external walls (protected from saturation)	FN, ML, MN	Class 3	Strength > 7 N/mm ²	
Un-rendered external walls (not protected from saturation)	FL, FN	Class 3	Strength > 15 N/mm ²	Use sulphate resisting cement in mortar with type N clay bricks
Rendered external walls	FL, FN, ML, MN	Class 3	Strength > 7 N/mm ²	Use sulphate resisting cement in mortar and base coat of render with type N bricks
Copings, cappings and sills	FL, FN	Class 4	Strength > 30 N/mm ²	
Internal	FL, FN, ML, MN, OL, ON	Class 3	All	

Table 5: Suitability of brickwork in masonry

Use	Designation	Proportion by volume			Minimum compressive strength (N/mm ²) ^(a)
		Portland cement: lime: sand	Air-entrained Portland Cement: Sand	Masonry cement: sand	
Mortar for internal and external use above DPC	(iii)	1:1:5-6	1:5-6	1:4-5	2.5
General purpose to BRE Digest 362		Air-entrained with plasticiser Portland cement: lime: sand 1:1:5.5 by volume			2.5
High durability mortar for A Use below or near external ground level B in parapets and chimneys C External walls with high risk of saturation due to severe weather exposure	(ii) ^(b)	1:0.5:4-4.5 ^(c)	1:3-4 ^(c)	1:2.5-3.5 ^(c)	5.0N/mm ²
If type N clay bricks are to be used, or for all chimneys use sulphate resisting cement					
Low permeability jointing mortar including copings, cappings and sills	(i) ^(d)	1:0.25:3 Use a Type S sand to BS 1200	N/A	N/A	N/A
Loadbearing masonry designed to BS 5628:1		Air entrained with plasticiser, Portland cement: lime: sand 1:1:5.5 by volume			As specified
Notes: ^(a) Minimum compressive strength ¹ of site mixed mortars at 28 days (N/mm ²) ^(b) For concrete or calcium silicate brick use a designation (iii) mortar ^(c) Where soil or ground-water sulphate levels are appreciable (Class 3 or higher) use sulphate resisting Portland cement. ^(d) For concrete or calcium silicate bricks use designation (ii) mortar					

Table 6: Suitability of mortar

2.3.4 Standards referred to

- BS 6399 Loadings for Buildings.
- BS 8103 Structural Design of Low Rise Buildings.
- BS 187: 1978 Specification for calcium silicate (sandlime and flintlime) bricks.
- BS 5628 parts 1, 2 and 3 Code of Practice for use of masonry.
- BS EN 998 Specification for mortar for masonry.